

Near-shore swell estimation from a global wind-wave model: spectral process, linear, and artificial neural network models

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Abstract

Estimation of swell conditions in coastal regions is important for a variety of public, government, and research applications. Driving a model of the near-shore wave transformation, from an offshore global swell model such as NOAA WaveWatch3, is an economical means to arrive at swell size estimates at particular locations of interest. Recently, some work (e.g. Browne et al. (2006)) has examined an artificial neural network (ANN) based, empirical approach to wave estimation. Here, we provide a comprehensive evaluation of two data driven approaches to estimating waves nearshore (linear and ANN), and also contrast these with a more traditional spectral wave simulation model (SWAN). Performance was assessed on data gathered from a total of 17 near-shore locations, with heterogenous geography and bathymetry, around the continent of Australia over a 7 month period. It was found that the ANNs out-performed SWAN and the non-linear architecture consistently out-performed the linear method. Variability in performance and differential performance with regard to geographical location could largely be explained in terms of the underlying complexity of the local wave transformation.

Key words: artificial neural networks, near-shore wave transformation, wave modeling, wave estimation

1 Introduction

Knowledge of swell conditions at specific nearshore locations is important for coastal research, marine engineering, and policy development. Although global swell models are an effective approximation of open swell conditions, they become less accurate in the nearshore zone. Many forms of remote sensing suffer from similar issues; satellite observations, for example, due to the manner in which they are sensed and averaged, normally pertain only to deeper ocean locations, typically 30km away from the the shoreline (Kalra et al., 2005). Variations in nearshore bathymetry, local wind-generated seas and the effects of artificial structures transform deep water swell due to reflection, shoaling, refraction, diffraction and breaking (Londhe and Deo, 2004). At a particular location, local topography may lead to attenuation or accentuation of long or short period swells, either directly, or by the contribution of local wind conditions. This makes the estimation of onshore wave heights, even given reliable offshore swell measurements, a non-trivial exercise.

The ability to estimate and predict onshore wave heights at the shore is of significant public interest in countries such as Australia, due to the high level of public activity in or near the break zone. There is a clear need for this

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19 data both as a general advisory to the public concerned with recreational use
20 of the near-shore zone, and for life-guards concerned with providing advice in
21 order to ensure public safety. Much of the Australian coastline is characterised
22 by both high density structures, and highly dynamic sediment evolution, and
23 monitoring of onshore wave heights is important for monitoring and managing
24 coastal development. Finally, scientific and engineering evaluations of partic-
25 ular locations and structures in this zone depend on reliable data on wave
26 action.

27 Currently, estimation of onshore wave heights from visual inspection is carried
28 out on a regular basis, and this information is broadcast through private and
29 commercial channels. The conventional approach is for this estimation to be
30 performed and recorded manually by human observers, using a combination
31 of heuristics, local knowledge, and offshore wind and wave models published
32 by government agencies. Numerical models based on wave propagation theory
33 have been neglected as a practical onshore wave height estimation tool due
34 to a combination of factors: complexity of implementation, high amounts of
35 processor time required, the need for accurate local bathymetric surveys, and
36 general inaccuracy, even when the previous conditions are met. However, there
37 is a clear need to substitute an objective and automated approach for human
38 observations of onshore wave activity, which are expensive, time consuming,
39 and prone to the usual forms of human failure and error.

40 In academic research, the propagation of swell in nearshore areas is conven-
41 tionally studied by running either an actual, or a virtual simulated physical
42 model (Londhe and Deo, 2004). Physical scale models require a significant
43 investment of resources for their construction and simulation. For this reason,
44 physical modeling using numerical computer simulation incorporating the lo-

cal physical environment and local swell conditions is often used. The Simulating Waves NearShore (SWAN) numerical model is an example of a popular approach for modeling wave propagation in the near-shore zone. However, numerical models themselves require care and expertise in their implementation. For example, a wave prediction system based on a numerical model must often contend with physical processes on a wide range of scales. These techniques are also sensitive to accuracy of the bathymetric data for the study area, and the quality of driving data at the model boundaries. Typically, hours of processing time is required for simulation of a region with adequate temporal and spatial resolution. Due to technical issues that may arise, results are sometimes unsatisfactory, especially in the break-zone.

Empirical wave recordings and observations are used to calibrate and validate theoretical (Booij et al., 1999a) and empirical (Komar and Gaughan, 1972; Caldwell, 2004) approaches to modelling the wave transformations that occur as they progress from deep to shallow water. The method of Caldwell (2004), for instance, was based on comparisons between buoy-measured H_s in deep water in close proximity to locations where visual observations were taken of trough to crest vertical wave height at the break point. There has been interest in developing empirical, data-driven models of nearshore wave characteristics for many years (Booij et al., 1999a). For example, an empirical method of obtaining surf height at the breakpoint from offshore wave data was derived from observations by Komar and Gaughan (1972). Following this approach, an alternative measure of surf height, H_{surf} , was developed for coastal zones with narrow shelves, steep bottom slopes and high refraction,

$$H_{\text{surf}} = H_b K_r(H_b), \tag{1}$$

70 where K_r is the empirical estimation of the refraction coefficient as a function
71 of shoaling and buoy-estimated breaker height, H_b :

$$72 \quad K_r = -0.0013H_b^2 + 0.1262H_b + 0.3025. \quad (2)$$

73 The method is based on comparisons between buoy-measured H_s in deep water
74 in close proximity to locations where visual observations were taken of trough
75 to crest vertical wave height at the break point. We note the particular oper-
76 ational definitions of wave height in the break zone (e.g. trough-crest height,
77 assessed visually): it should be recognised that it is usually impossible to mea-
78 sure breaking wave height using a fixed wave gauge, since the spatial point
79 of breaking varies with incident wave conditions. Estimating near-shore wave
80 height through modelling the shallow-water wave transformation via simpli-
81 fied equations that are optimised using real-world data may be considered a
82 'semi-empirical' approach.

83 In their application to analysis in the physical sciences, artificial neural net-
84 works (ANNs) can be regarded as strongly empirical, as opposed to model-
85 based approaches to estimate and predict wave behaviour (Deo and Jagdale,
86 2003). ANNs are a flexible learning architecture which rely on the presentation
87 of input and target data, rather than a theoretical model, for the estimation
88 of an underlying physical relationship. As general purpose function approxi-
89 mators, ANNs purposely impose no constraints on the final model generated,
90 although the size of the neural network necessarily limits the overall complex-
91 ity of the modelled function. In their role of associating temporally, spatially,
92 or modally distinct measurements of wind-wave activity, they must approxi-
93 mate the physical propagation of wave energy. The effect of local geography
94 or bathymetry is inferred from the co-variation of input-target pairs, rather

95 than explicitly determined. Thus, a representative corpus of training data is
96 essential for the function approximation potential of an ANN to be realised.

97 The use of ANNs has been reported for numerous applications in the geo-
98 logical and marine sciences, and in particular have been used for forecasting
99 wave climate time series (Deo et al., 2001; Agrawal and Deo, 2002). Tsai and
100 Lee (1999) utilised neural networks for forecasting tidal variability and Tsai
101 et al. (1999) used neural networks for forecasting wave heights at near-shore
102 locations, using measurements from other locations as input, finding that us-
103 ing multiple sites as input increased the accuracy of predictions. Some work
104 has tested ANNs for specific oceanic structural and engineering tasks: (Mase
105 and Kitano, 1999) used feed-forward networks to estimate wave force impact
106 on a marine structure, and (Mase et al., 1995) found that a similar ANN ar-
107 chitecture accurately predicted damage levels on a breakwater resulting from
108 wave action. ANNs have recently shown to produce superior estimates of wave
109 spectra from wave parameters than those provided by theoretical or statistical
110 predictions (Naithani and Deo, in print, 2005). Non-linear empirical models
111 have been shown to approximate well the underlying wave physics (Tolman
112 et al., 2005).

113 ANNs have been applied to estimating missing wave buoy data (Balas et al.,
114 2004), and recently Kalra et al. (2005) have detailed an ANN-based effort to
115 map offshore wave data to coastal locations, reporting superior performance
116 of ANNs compared to a linear statistical approach. The authors estimated
117 wave activity in the near-shore region from satellite monitoring at offshore
118 locations. Scotto and Soares (2000) concluded that in estimating significant
119 wave height of sea-states using non-linear autoregressive (AR) models, linear
120 models satisfactorily modelled the lower statistical moments, but non-linear

models better approximated the higher moments such as skewness and kurtosis. Also using an AR model, Ho and Yim (2005), recently demonstrated the feasibility of interpolating missing buoy data between two wave measuring stations.

Neural networks have been demonstrated to be superior to conventional approaches for forecasting significant wave height in open water, in a variety of situations (Deo et al., 2002, 2001; Deo and Kumar, 2000; Balas et al., 2004). Rao and Mandal (2005) focused on ANNs as an alternative to numerical modelling for estimating wave-fields generated by cyclone events. Browne et al. (2006) emphasised the use of ANNs for bridging of modes of observation (i.e. global model output / logged buoy data / visual observations) in the context of bringing offshore estimates to estimate activity in the near-shore zone. It has been shown that non-linear ANN approaches outperformed linear statistical and numerical modelling, in the estimation of both human observations at the beach, and wave-rider buoy data.

Apart from the welcome comparison of ANN performance with that of linear regression by Kalra et al. (2005), there have been insufficient comparisons of ANNs with other forms of swell estimation, such as linear predictors, and numerical modelling. Work reported by Browne et al. (2006) represented a first step towards a systematic comparison between ANNs, statistical, and numerical modelling approaches. However, this work needs to be extended to include a greater variety of geographical areas, and longer study time periods.

As neural networks are unconstrained general-purpose function approximators, with potentially thousands of degrees of freedom, care must be taken in their application in order to ensure that the underlying function is in fact well

146 approximated, allowing in turn for good generalisation to new data. *Over-*
147 *fitting*, which results from a high ratio of model degrees-of-freedom to pre-
148 sented data, is a particularly common pitfall for engineering and scientific ap-
149 plications of ANNs. These issues have led to concerns being raised regarding
150 the validity of previous work in applying ANNs to wave estimation. For exam-
151 ple, a method of wave forecasting using neural networks was recently reported
152 by Makarynskyy (2004), and subsequently challenged by Medina (2005) due to
153 issues related to over-fitting, lack of baseline performance comparisons, and an
154 insufficient degree of cross-validation. Inappropriate application of ANNs of-
155 ten leads to spuriously high performance estimates. It must be also noted that
156 as a strongly empirical approach, ANNs do not provide the insight into wave
157 propagation processes that is provided by full-scale numerical modelling. How-
158 ever, the advantages include computational efficiency and potentially greater
159 predictive power, without the need for detailed geographic information, or the
160 laborious testing of a range of physical model parameters.

161 Recent research is increasingly focusing on the use of neural networks in the
162 role of an unconstrained empirical function approximation tool for estimating
163 the relationship between geographically or temporally displaced observations
164 of wave height. However, to-date little attention has been paid to applying
165 them to approximate the deep-to-shallow water wave transformation. The
166 current paper is focused on mapping offshore global wind-wave observations
167 to activity observed at particular onshore locations. This requires the model
168 to incorporate the effects of a number of physical processes such as bottom
169 friction, diffraction and refraction, generated by an interaction of local geog-
170 raphy and bathymetry with the offshore wave field. Recent studies by Kalra
171 et al. (2005) and Browne et al. (2006), have reported superior performance

172 by ANNs. However, each study considered only a single geographical location
173 for a relatively short time, which prevented strong conclusions regarding the
174 relative efficacy of ANNs to be drawn.

175 The present study attempts definitive testing of ANNs for modeling the nearshore
176 wave transformation: bringing global ocean wave model output to nearshore
177 locations, and demonstrating a potentially useful tool for emulating expensive
178 surf reporter observations. A comprehensive evaluation of empirical methods is
179 attempted by considering a total of 17 onshore locations across 5 geographical
180 regions distributed across the continent of Australia, for a period of 8 months.
181 ANNs are compared with baseline, linear and model-based approaches and
182 explanations for the differential performance are provided. Detail is provided
183 on the technical implementation and validation of ANN performance, which as
184 discussed above, is critical for establishing an accurate benchmark of perfor-
185 mance. The SWAN numerical model is applied for performance comparisons,
186 and in order to investigate the characteristics of the study areas.

187 **2 Method**

188 *2.1 Study Regions*

189 As noted above, in order to achieve a comprehensive evaluation of spectral,
190 linear, and ANN based modeling, the present study considered five regions
191 in Australia, each with distinct properties, in both in terms of bathymetry
192 and wave climate. This is illustrated in the regional maps (Figs 2,3 & 4). The
193 red filled circles denote the onshore locations considered in this study, while
194 the red stars indicate the theoretical location of the NOAA WW3 grid point.

195 Figure 2 maps the bathymetry of each region, which shows a range of profiles,
 196 from open, exposed beaches, to sheltered bays, along with a range of island
 197 and headland features. Figure 3 plots the mean wave height, as estimated by
 198 the SWAN model from the NOAA data over the time period of interest: this
 199 may be considered in conjunction with table 2 to provide a view of regional
 200 and intra-regional wave climate variability. Finally, figure 4 shows the degree
 201 of linear relationship (correlation r) of the SWAN model output at each point
 202 within each region, to that region’s NOAA WW3 grid point, over the study
 203 period. Lighter values indicate a smaller proportion of the variability being
 204 explained by a linear relationship with offshore conditions, and hence indicate
 205 a greater proportion of non-linear wave effects in the SWAN model output
 206 at that point. We consider this to be a useful guide to the model-estimated
 207 complexity in the wave-transformation over each locale with each study region.

208 2.2 Data

209 In the following sections we describe the various forms of data used in this
 210 study; bathymetric surveys used as input to the numerical physical model,
 211 driving variables from NOAA WaveWatch 3 (NWW3), and visual surf reporter
 212 estimates of H_s .

213 2.2.1 NOAA WaveWatch 3

214 The driving variables for all models tested in this study were drawn from
 215 global wave model data gathered from the NWW3 model at 6 grid locations in
 216 various regions off the coast of Australia from 12/01/05 to 9/8/05. The NWW3
 217 model provided updates four times daily, at 4am, 12pm (midday), 6pm, and

218 12am (midnight). The NWW3 variables were; significant wave height, primary
 219 swell direction, primary swell period, wind direction, wind speed, secondary
 220 swell direction, secondary swell period, wind wave direction, and wind wave
 221 period. NWW3 generates swell forecasts at every three hours from +3 to +180
 222 hours ahead of the current time as well as a single analysis at 0 hours, which
 223 represents the model state, given current measured data. In the present study,
 224 only the 0 hour state information was used. For the purpose of presenting the
 225 NWW3 variables to the ANNs for empirical prediction, variables in degree
 226 format present an issue because of the discontinuity at the $0^\circ/360^\circ$ point.
 227 Each of the directional variables; primary swell direction, wind direction and
 228 wave direction were transformed to Cartesian co-ordinates ('northerliness' D^N
 229 and 'easterliness' D^E):

$$230 \quad D^N = \cos\left(\frac{\pi D}{180}\right), D^E = \sin\left(\frac{\pi D}{180}\right), \quad (3)$$

231 The locations of the NWW3 grid points are shown in table 2. Table 2 displays
 232 the cross-correlation matrix between H_s at the various grid points. As ex-
 233 pected, there are correlations between nearby grid points; i.e. between Queens-
 234 land (QLD) and North New South Wales (NNSW) regions, and between the
 235 South Australia (SA) and Victorian (VIC) grid points. However, 11 of the 15
 236 cross-correlations are below 0.3, indicating that overall, the swell conditions
 237 around the continent had a significant degree of statistical independence dur-
 238 ing the study period. This entails that to a large extent, although the regions
 239 were considered over the same time period, the study regions represent inde-
 240 pendent sources of testing the estimation methods. The univariate statistics
 241 of the NWW3 estimated H_s in table 2 also illustrate the heterogeneous nature
 242 of the offshore swell conditions around Australia. Southern areas (i.e. VIC,

SA and Western Australia (WA)) had higher and more variable seas, whilst NNSW and QLD were characterised by greater skewness in H_s , primarily due to the large contribution that intermittent tropical storm events make to the overall variability of H_s in these areas.

2.2.2 Bathymetric Data

Bathymetric data is a key parameter to undertake nearshore wave modelling. Bathymetric data were required for 5 areas of the Australian Coast from deep water to shallow water in order to compute the wave propagation from the NWW3 output grid points to the surf reporter locations. The publicly available Geoscience Australia bathymetric database (www.ga.gov.au) was used to generate the whole bathymetry of 4 areas.

The database contains data around the Australian margin since 1963 from a variety of systems and levels of accuracy. A total of 931 surveys are currently registered, whose extent is 34N - 79S, 90 - 180E. The typical point data spacing is 25-200 m. Approximately 20% of these surveys were acquired by Geoscience Australia, the other component being surveys from other institutions, such as oil exploration companies and the National Geophysical Data Centre to which various institutions have contributed. Swath bathymetry in deep water, laser airborne depth sounder (LADS) data, points digitised from Australian Hydrographic Service charts on the shelf and predicted bathymetry from satellite altimetry (Smith and Sandwell, 1997) have been brought together. The database grid was computed at a cell size of 0.01 (36" or 1111 metres) as a compromise between conveying detail and limiting the file size.

For the Gold Coast area, accurate bathymetry data were provided by the Gold

267 Coast City Council. ETA lines covering the Gold Coast from the Gold Coast
268 Seaway to Tweed Heads and refined bathymetric surveys around Burleigh
269 Heads and Currumbin Creek estuary were combined with the Geoscience Aus-
270 tralia database for the offshore information (typically used from 20 to 80 m
271 depth).

272 2.2.3 *Surf Reporter Data*

273 In collaboration with the Australian surf monitoring firm CoastalWatch™,
274 the study relied on a network of professional surf reporters for performing daily
275 visual estimates of significant wave height at each of the 17 beach locations.
276 Estimates were made using a mobile internet link and time-stamped. Visual
277 inspection was usually performed once per day, between 6am and 8am, but
278 during dynamic periods a second visual estimate was made in the afternoon.
279 Table 1 displays the number of observations and number of days for which
280 observations were recorded for each of the 17 beaches. As the surf reports
281 were the limiting source of data for optimizing the models, this table also
282 summarizes the number of input / target data pairs available for analysis.
283 Table 3 displays the univariate statistics for the surf reporter observations at
284 each beach study site. As expected, both overall significant wave height and
285 variability are significantly less than the open ocean estimates produced by
286 NWW3. With reference to the regional maps and the NWW3 statistics, it may
287 be seen that there is variability between the various study areas corresponding
288 to both offshore swell characteristics between regions, and local geographic
289 features. At certain nearby beaches, where local geography and bathymetry
290 is relatively homogeneous, there is a high degree of similarity between the
291 univariate statistics of the beaches. This is supported by table 4, which displays

the normalised bivariate correlations in H_s between the surf reporter sites. The Sydney, New South Wales (NSW) beaches in particular, which are grouped in a relatively small region in and around Sydney, and similar exposure to offshore swell, have a high degree of inter-dependence. In general however, there is wide variability in the univariate moments due to the different swell conditions prevailing in the different continental locations, and considerable variety in local geography and bathymetry.

2.3 Analysis and Simulation

2.3.1 ANN Background

ANNs are widely accepted as a valuable tool for modeling, approximation, and classification (Kostanic (2001); Bishop (1995); Ripley (1996)). The common fully inter-connected feed-forward architecture implements a mapping $\mathbf{y} = f(\mathbf{x}) : \mathfrak{R}^m \rightarrow \mathfrak{R}^n$ and is optimized by providing multiple (assumed noisy) paired samples of the input and target output $\{\mathbf{i}_p \in \mathfrak{R}^m, \mathbf{t}_p \in \mathfrak{R}^n\}$. The transfer function g , which generates a unit's output given net activation from connections to units in the previous layer, should generally be smooth and have a well bounded range for any input; e.g. $g : (-\infty, \infty) \rightarrow (-1, 1)$ for the tan-sigmoidal case (logistic and gaussian functions are also common basis functions). Assuming the activation function at each layer $i = \{1, \dots, L\}$ is homogenous, an ANN implements the function

$$f(\mathbf{x}) = f(\mathbf{a}_0) = h_L(h_{L-1}(h_{L-2}(\dots(h_1(\mathbf{a}_0)))) \quad (4)$$

313 with the layer transformation h defined by

$$314 \quad \mathbf{a}_i = h_i(\mathbf{a}_{i-1}) = g_i(\mathbf{W}_i [\mathbf{a}_{i-1}^T \ 1]) \quad (5)$$

315 where the set of free parameters (termed weights) in the system $\{\mathbf{W}_i\}$ deter-
 316 mine the particular non-linear mapping, noting that $\mathbf{a}_0 = \mathbf{x}$ is an m element
 317 vector, $\mathbf{a}_L = \mathbf{y}$ an n element vector, and necessarily the dimensions $\{d_i^1, d_i^2\}$
 318 of \mathbf{W}_i have the constraints $d_1^1 = m + 1$, $d_L^2 = n + 1$, and $d_i^1 = d_{i-1}^2$. This
 319 notation represents the constant or *bias* term as a unit input appended to
 320 the output vector of the previous layer. ANNs are usually conceptualised as
 321 a series of neural layers, with forward interconnections between subsequent
 322 layers, as shown in figure 1.

323 A common application of feed-forward networks for estimation involves a fixed
 324 architecture or topology, with two or three layers L , and an arbitrary num-
 325 ber of neurons (defined by d_i^2) in each layer. Training a neural network usu-
 326 ally involves minimising an error function (e.g. for mean-square error (MSE),
 327 $\epsilon_n = \sum_p (\mathbf{y}_p - \mathbf{t}_p)^2$), utilising local gradient search algorithms operating on
 328 $-\delta\epsilon_n/\delta\{\mathbf{W}_i\}$, the derivative of the error function with respect to the free
 329 weight parameters in the network. Sophisticated and efficient search algo-
 330 rithms, such as the Levenburg-Marquardt method or conjugate gradient de-
 331 scent (Marquardt, 1963; Kan and Timmer, 1989), along with modern com-
 332 putational resources, allow for fast optimisation of medium sized networks.
 333 Single layer, or linear feed-forward networks converge to a global optimum.
 334 The function-approximation power of non-linear ANNs with one or more hid-
 335 den layers is based on the non-linearity of the basis functions in the hidden
 336 layers. However, this property also entails the presence of *local minima* in the
 337 error function. ANN implementation requires acknowledgement that optimi-

sation based on local gradients may be expected to yield solutions located in some form of local minima: i.e. good but not optimal solutions. Multiple runs with random initial $\{\mathbf{W}_i\}$ are a straight-forward way to alleviate this issue. Further, since ANN architectures can involve a large number of free parameters, *over-fitting* of the training data is common: this must be taken into account both in model optimization, and in estimating model effectiveness.

2.3.2 ANN Implementation

In the present study, the basic feed-forward ANN architecture was used to implement three empirical estimates of surf reporter readings of significant wave height, H_s^{sr} . In each case, the 9 NWW3 parameters (given in 2.2.1) were used as inputs to the model, with the four directional angle variables transformed to 2D Cartesian co-ordinates on the unit circle, leading to a total of 13 inputs. Firstly, a simple linear scaling of NWW3 derived H_s , H_s^N was implemented:

$$\widehat{H}_s^{sr} = w_1 H_s^N + w_0 \quad (6)$$

with the model weights w determined empirically. Because of the extreme simplicity of this model, we refer to this as the baseline-scaled estimate. In order to reduce the number of input variables, and hence the number of free weights in the networks, principle components analysis (PCA) was applied as a pre-processing step ⁴. Using a form of scree plot, it was decided that retaining the first 6 orthogonal projections of the data retained a reasonable level (> 92%) of total (normalised) variability in the NWW3 data. Given the vector

⁴ As PCA is a standard data pre-processing technique for reducing data dimensionality, it will not be discussed. For more details the reader is directed to Jolliffe (2002).

360 of normalised NWW3 driving variables \mathbf{n} , and the 13x6 PCA transformation
 361 matrix \mathbf{P} , the linear empirical model is given by

$$362 \quad \widehat{H}_s^{sr} = \langle \mathbf{w}^T, [\mathbf{n}^T \mathbf{P} \ 1]^T \rangle \quad (7)$$

363 where $\langle \cdot \rangle$ denotes the inner product operation, and the 6-element weight vec-
 364 tor \mathbf{w} is optimized with respect to the training data. All feed-forward ANN
 365 and non-ANN model optimization was done using the Levenburg-Marquardt
 366 (LM) algorithm implemented in MATLAB. Like other iterative optimization
 367 algorithms, the LM method finds parameter values that minimize the sum
 368 of squares using local gradient information of the objective function. It is a
 369 more robust form of Gauss-Newton algorithm (which utilizes first derivative
 370 information in estimate updates).

371 The final empirical model is a non-linear feed-forward neural network utilising
 372 tan-sigmoid activation functions

$$373 \quad h(x) = \frac{2}{1 + e^{-2x}} - 1 \quad (8)$$

374 in the hidden layer, and linear activation in the output layer. The non-linear
 375 ANN model used a single hidden layer of 6 neurons, the equation being:

$$376 \quad \widehat{H}_s^{sr} = \left\langle \mathbf{w}_2^T, \left[h([\mathbf{n}^T \mathbf{P} \ 1] \mathbf{W}_1) \ 1 \right] \right\rangle \quad (9)$$

377 with the 6x7 input-to-hidden connection matrix \mathbf{W}_1 and the 1x7 hidden-to-
 378 output weight vector \mathbf{w}_2^T being determined empirically.

379 The multi-layer ANNs used in the study were purposely made as small as
 380 possible in order to reduce the potential for over-fitting the data: the non-
 381 linear architecture involved 49 free weights. It was assumed that a reasonable

382 approximation to the shallow-water transformation for a single location would
 383 not require an overly complex model. The available data for the surf reporters
 384 at each onshore location varied from 800 to 900 data points. The ANNs were
 385 trained using a 5-fold combined early-stopping and cross-validation method.
 386 That is, 80% of the data was used for optimising the free weights in the
 387 model, with 20% used for cross-validation of the data, and early stopping of
 388 training. This approach is intended to prevent over-fitting of the training data,
 389 and for generating reliable estimates of performance on unseen data. For the
 390 non-linear model given in (9), each partition of the data was used to train
 391 10 ANNs with identical topology, but different random weight initialisations,
 392 and the best performing neural net used in each case. Finally the *ensemble*
 393 ANN output was calculated by averaging the output of the 10 ANNs upon
 394 presentation of the entire data set. The ensemble method is known to improve
 395 performance and generalisation of ANNs, but the performance in this case is
 396 known to be an overestimate, as the performance calculation is necessarily
 397 performed on the combined training and test data subsets for all the trained
 398 neural networks. Therefore, in tables 5 and 6, the validation performance
 399 column may be treated as a conservative estimate of future ensemble ANN
 400 performance, whilst the reported ANN ensemble performance is rather more
 401 optimistic.

402 2.3.3 *SWAN Background*

403 SWAN (version 40.41) is a spectral wave model based on the action density
 404 balance equation (Hasselmann et al., 1973) that describes the evolution of two-
 405 dimensional wave energy spectra under specified conditions of winds, currents,
 406 and bathymetry (Booij et al., 1999b; Ris et al., 1998).

407 SWAN can be used on any scale, even if this model is specifically designed
408 for coastal applications. This model requires no restriction on wave approach
409 angle or directional width. SWAN is able to simulate accurately the wave field
410 in coastal areas where reflection and diffraction are not significant. The SWAN
411 Cycle III User Manual by Holthuijsen et al. (2002) provides a detailed account
412 of the theoretical background, program structures, and implementation.

413 SWAN modeling has been used successfully for storm-induced coastal flooding
414 assessment applications (Cheung et al., 2003), to drive nearshore wave-induced
415 current nearshore models (Castelle et al., 2005), for wind generated waves in
416 lakes (Jin and Z.-G., 2001) and in coastal regions (Ou et al., 2002; Castelle
417 et al., 2006), and to model the evolution of wave spectra in a wave tank (Wood
418 et al., 2001).

419 2.3.4 *SWAN implementation*

420 In the present study, SWAN is used in stationary mode. The model considers
421 a steady state situation that requires the time of propagation of the waves
422 through the domain to be short compared to the variation of water level,
423 currents and changes in offshore wave conditions. Triad interaction is taken
424 into account in the computations. The breaking wave model chosen herein is
425 the bore-based model of Battjes and Janssen (1978), with a constant breaker
426 parameter $\gamma = 0.73$ following Battjes and Stive (1985). The wave forcing
427 provided by the NWW3 nearest output point is to the offshore and lateral
428 boundaries of the model. For Western Australia, South Australia, Victoria
429 and New South Wales areas, the computational grid is concurrent with the
430 Geoscience Australia grid i.e. a regular grid at a cell size of 0.01. For the

431 Queensland area, a curvilinear grid is used for computations with grid cell
 432 size of $O(100 \text{ m})$. During the simulations, the wave information is requested
 433 at each surf reporter location. The outputs are given in 10 m depth in order
 434 to avoid an underestimation of wave height due to wave breaking during high
 435 energy conditions. The tide level is treated as constant equal to 0 in the
 436 Admiralty Height Datum (AHD), i.e. at mid tide. Stationary computations
 437 are done every 6 hours, concurrent with the NWW3 output data.

438 **3 Results**

439 Table 5 presents the performance of the various empirical models in terms
 440 of mean absolute error (MAE) for the 17 surf reporter locations, organised
 441 according to study area. A single NOAA WW3 grid point served as input to
 442 models for all the surf reporter locations in each study area. For ease of pre-
 443 sentation, the MAEs are provided in cm rather than metres. The first column
 444 displays the difference between the raw significant wave heights H^s recorded at
 445 the offshore NOAA grid point and the surf reporter observations. This base-
 446 line difference ranged from approximately 0.6m at the Sydney, New South
 447 Wales beaches, to approximately 2.4m at Trigg Beach, WA and Seaford, SA.
 448 The next column displays the MAE after linear scaling of offshore H^s using
 449 eq. (6). At all locations, linear scaling resulted in a significant decrease in the
 450 MAE. This baseline measure represents the error after linear attenuation of
 451 wave energy is accounted for. Because of the insignificant degrees of freedom
 452 for this model (i.e. 1), division of the data into training and validation subsets
 453 was not performed. For the linear and non-linear ANNs (eq.(7) and (9)), data
 454 was split into training and validation and test sets, and the results of both

are shown. The incremental improvement of the linear and non-linear ANNs varied over the test sites. The ensemble ANN performance is also shown. Two sample t-tests ($N=5$) were performed in order to explicitly test the hypothesis that the non-linear ANN MAE was less than the linear version. Despite relatively small incremental improvements in performance in some cases, the t-statistics were significant at the .01 probability level for all locations except for Warranambool, VIC.

Table 6 shows similar results for the normalised correlation coefficient R between empirical model estimates and surf reporter observations. The R coefficient provides a more sensitive measure of estimation performance. For consistency, raw and scaled correlations are included, although they are identical due to the fact that R is normalised with respect to univariate variance, and linear scaling therefore has no effect of the observed R . Here the improvement in estimation performance over the baseline is more apparent, and so too is the differential improvement over the various locations. Despite the greater degrees of freedom in the non-linear versus linear models, generalisation performance, as measured by the validation columns was consistently better for the non-linear networks. This is confirmed by the t-statistics, which indicate significant improvement in performance in almost every case.

Figures 5-9 compare graphically the output of the best performing non-linear neural network with the surf reporter observations. A time series over the study time period is shown on the left hand side of each plot. The two error bar plots on the right hand side summarise the relationship between either H^s offshore baseline (middle plot) or ANN network estimated H^s (right plot). In both cases, the diagonal line represents H^s as reported by the human observer, whilst the mean and standard error at each wave height is summarised by the

481 error bars.

482 The full page illustrations provided in figure 2 shows the bathymetric profiles
483 for each of the study regions for which bathymetry was available.

484 4 Discussion

485 We first consider the effectiveness of SWAN modeling for nearshore wave es-
486 timation. For each area, there is a significant improvement of the wave es-
487 timation in terms of the Mean Standard Error (MAE) at the surf-reporter
488 locations in comparison with the NOAA outputs. Thus, the overall degree of
489 wave bottom friction over the continental shelf and wave energy shadowing
490 behind headlands and islands appears to be correctly estimated. However,
491 the results are generally not comparable with either the linear or non-linear
492 empirical models.

493 The effectiveness of SWAN modeling is, not surprisingly, more effective when
494 accurate bathymetry are available and a more refined computational grid is
495 used. In the Queensland area, for which this was the case, the correlation
496 factor significantly improves in comparison with NOAA output data (from
497 0.49 to 0.72), as does the Mean Standard Error (0.94 m to 0.32 m). This area
498 is exposed to moderate to high energy southerly to south-easterly swell during
499 the winter period. During these wave conditions, the surf reporter location is
500 partially sheltered by the Coolangatta headlands. This sheltering and resulting
501 wave refraction patterns are mostly responsible for the significant improvement
502 of the correlation factor.

503 For the other areas, SWAN performance in terms of the correlation statis-

504 tics is significantly worse in comparison to the ANN estimators, and even
 505 the baseline. This can be explained by many factors. The main concern is
 506 the bathymetric data, and consequently the coarse computational grid. In the
 507 Queensland region, comparison of Geoscience Australia bathymetric data and
 508 bathymetric surveys provided by the Gold Coast City Council showed that
 509 errors of the order of sometimes a few meters could be measured. The main
 510 processes affecting the wave propagation exist in the nearshore zone, usually
 511 between 30-40 m depth. For example, in the New South Wales area, there
 512 is significant longshore variability of the seabed between 0 and 50 m depth
 513 which are poorly revealed by Geoscience Australia database. Mid-scale pro-
 514 cesses such as refraction are poorly simulated by SWAN in the present study.
 515 Furthermore, for areas with a large and shallow continental shelf such as the
 516 Western Australia area, calibration of bottom friction is a key parameter for
 517 an accurate estimation of wave height to the shore. Finally, one of the main
 518 reasons why SWAN results are not in very good agreement with surf reporter
 519 wave height estimation is the wave forcing format. Indeed, significant wave
 520 height, mean period and mean wave angle are provided by NWW3 outputs
 521 in our areas of interest. Directional wave spectra, which were not available
 522 in the present study, are necessary for optimal performance of model-based
 523 estimation techniques.

524 The ANN based empirical method used, on the other hand, does not depend
 525 on careful adjustment of physical parameters (such as bottom friction) and
 526 handles gracefully sub-optimal input data (i.e. swell parameters in lieu of
 527 the complete directional spectrum). It was expected that the non-linear ANN
 528 architecture would perform better than the linear models, and this was borne
 529 out by the results. Tables 4 and 5 show that non-linear networks consistently

530 out-performed linear networks over the study regions both in terms of standard
531 error, and correlation with the surf reporter targets. This need for a non-linear
532 model was in accordance with predictions, as a non-linear model was expected
533 to be necessary to take into account the interaction of variable offshore forcing
534 conditions and complex local bathymetries. From figures 2-4, it is clear that a
535 significant proportion of the shared variability in offshore and onshore observed
536 swell heights is modulated by non-linear physical processes in the form of
537 refraction, wave interactions, and local wind effects.

538 The simultaneous prediction of near-shore wave heights over a large number
539 of study regions, along with the use of SWAN modeling of wave propagation
540 in the region (figures 2-4) allows us to consider the reasons for differential pre-
541 diction performance in each study area. The Queensland region, for instance,
542 is subject to variability in swell direction, biased towards the south-east. For
543 these beaches, the relationship between offshore and onshore swell height is
544 moderated strongly by swell direction, with more southerly swells experiencing
545 a higher degree of sheltering and refraction. In the case of Seaford SA, both lin-
546 ear and non-linear ANNs failed to markedly improve onshore swell estimation.
547 This may be explained by the fact that offshore and onshore activity is highly
548 decoupled: the highly dynamic offshore wave climate has limited propagation
549 to the onshore site due to sheltering. Poor performance at this site also results
550 from the inherent difficulty in monitoring small changes in wave height, and
551 the fact that a higher proportion of this variability is due to unpredictable
552 effects such as localized winds. The utility of non-linear estimation was more
553 apparent at Chiton Rocks SA, which may be explained by its more exposed
554 aspect, yielding a more consistent relationship between offshore and onshore
555 activity, which the empirical models were able to emulate. A non-linear model

556 was more effective because it was able to take into account differential attenu-
557 ation due to swell direction. The empirical estimation methods were less useful
558 on Sydney (SNSW) beaches, where the baseline relationship between NOAA
559 and the surf-reporters was very high. The lack of complexity in the offshore-
560 onshore wave transformation meant that there was little further improvement
561 that the linear or non-linear models could make on the baseline prediction.
562 The South Coast NSW study site was a slight exception to this rule, that we
563 explain by its greater distance from the NOAA grid point (it is located further
564 south, off-map). The best performance overall, in terms of ANN improvement
565 over baseline, was observed at Margaret River and Trigg beach, WA. This is
566 consistent with the general relationship observed between local site character-
567 istics and the improvement of estimation performance over baseline. That is,
568 non-linear ANNs are most effective for estimation when there is a systematic
569 yet complex coupling of offshore and onshore wave climates. Best performance
570 is observed when local transformative processes such as shoaling, attenuation
571 and refraction moderate, but do not completely decouple, the onshore from
572 the offshore wave climate. ANNs tended to over-estimate smaller swells. We
573 believe this is due to the fact that the ANN models assume a Gaussian error
574 distribution. However, this is unrealistic since; a) it is impossible to observe
575 negative wave heights, and b) the wave height distribution is positively skewed.
576 This could be addressed in future work by utilising a zero-truncated, or quasi-
577 binomial distribution.

578 5 Conclusions

579 The methodologies considered here have immediate application for near-shore
580 wave height estimation. This is significant because the vast majority of hu-
581 man activity occurs in the near-shore zone, and swell conditions in this area
582 are therefore of greatest interest to coastal managers and the public. When
583 combined with a near-shore model, existent global wave models such NWW3
584 can provide a reliable and cost-effective source of offshore data in absence
585 of *in situ* measurements. Compared with linear and spectral modeling, this
586 study has concluded that near-shore conditions can be inferred from WW3
587 parameters most effectively using ANN-based estimation. As well as explicit
588 and implicit modeling of the near-shore wave transformation, the paper es-
589 tablishes quite strong relationships between a open ocean global wind-wave
590 model and onshore visual estimates of wave height provided by experts. The
591 empirical approach presented here relies on the availability of a corpus of tar-
592 get data for model training: the use of a nation-wide database of expert surf
593 reporter records is a unique characteristic of this study. The practical appli-
594 cation rests on the ability to replace the manual ratings, with the automatic
595 estimates generated by the ANN model. The high correlations and relatively
596 small standard-errors obtained by the ANN model on the *validation* data set
597 indicates that 6-12 months of daily observations is sufficient to build a model
598 that generalizes well.

599 References

600 Agrawal, J. D., Deo, M., 2002. Online wave prediction. Marine Structures 15,
601 57–74.

602 Balas, C. E., Koc, L., Balas, L., Sept/Oct 2004. Predictions of missing wave
603 data by recurrent neuronets. Journal of Waterway, Port, Coastal and Ocean
604 Engineering, 256–265.

605 Battjes, J., Janssen, J., 1978. Energy loss and set-up due to breaking of random
606 waves. In: Proceeding of the 16th International Conference on Coast. Eng.
607 ASCE, pp. 569–587.

608 Battjes, J., Stive, M., 1985. Calibration and verification of a dissipation model
609 for random breaking waves. J. Geophys. Res. 90 (C5), 9159–9167.

610 Bishop, C., 1995. Neural Networks for Pattern Recognition. Clarendon Press.

611 Booij, N., Ris, R., Holthuijsen, L., 1999a. A third-generation wave model for
612 coastal regions. 1. model description and validation. J. of Geophys. Res.
613 104 (7), 649–666.

614 Booij, N., Ris, R., Holthuijsen, L., 1999b. A third-generation wave model for
615 coastal regions, part i: Model description and validation. J. Geophys. Res.
616 104 (C4), 7649–7666.

617 Browne, M., Strauss, D., Castelle, B., Lane, C., Blumenstein, M., 2006. Em-
618 pirical estimation of nearshore waves from a global deep-water wave model.
619 IEEE Geoscience and Remote Sensing Letters, in print, to appear 2006.

620 Caldwell, P. C., 2004. An empirical method for estimating surf heights from
621 deep water significant wave heights and peak periods in coastal zones with
622 narrow shelves, steep bottom slopes, and high refraction. In: Proceedings
623 of the 8th International Workshop on Wave Hindcasting and Forecasting.
624 North Shore, Oahu, Hawaii. November 14 -19.

625 Castelle, B., Bonneton, P., SÈnÈchal, N., Dupuis, H., Butel, R., Michel, D.,
626 2005. Dynamics of wave-induced currents over an alongshore non-uniform
627 multiple-barred sandy beach on the aquitanian coast, france. Cont. Shelf
628 Res. In press.

629 Castelle, B., Cope, M., Abbs, D., Tomlinson, R., 2006. Wave modelling for the
630 gold coast: down-sizing from global to nearshore. In: 13th National Confer-
631 ence: Climate, Water and Sustainability, Newcastle, Feb 6-8. The Australian
632 Meteorological and Oceanographic Society.

633 Cheung, K., Phadke, A., Wei, Y., Rojas, R. D.-M., Martino, C., Houston, S.,
634 Liu, P.-F., Lynett, P., Dodd, N., Liao, S., Nakasaki, E., 2003. Modeling of
635 storm-induced coastal flooding for emergency management. *Ocean Eng.* 30,
636 1353–1386.

637 Deo, M., Godane, D., Kumar, V. S., 2002. Analysis of wave directional spread-
638 ing using neural networks. *Journal of Waterways, Port and Coastal Engi-
639 neering* 128 (1), 30–37.

640 Deo, M., Kumar, N. K., 2000. Interpolation of wave heights. *Ocean Engineer-
641 ing* 27, 907–919.

642 Deo, M. C., Jagdale, S. S., 2003. Prediction of breaking waves with neural
643 networks. *Ocean Engineering*, 30, 1163–1178.

644 Deo, M. C., Jha, A., Chaphekar, A. S., Ravikant, K., 2001. Neural networks
645 for wave forecasting. *Ocean Engineering* 28 (7), 889–898.

646 Hasselmann, K., Barnett, T., Bouws, E., Carlson, H., Cartwright, D., Enke,
647 K., Ewing, J., Gienapp, H., Hasselmann, D., Kruseman, P., Meerburg, A.,
648 Miller, P., Olbers, D., Richter, K., Sell, W., Walden, H., 1973. Measurements
649 of wind-wave growth and swell decay during the joint north sea wave project
650 (jonswap). *Dtsch. Hydrogr. Z.* 12 (A8).

651 Ho, P., Yim, J. Z., 2005. A study of the data transferability between two
652 wave-measuring stations. *Coastal Engineering* 52, 313–329.

653 Holthuijsen, S., Booij, N., Ris, N. Haagsma, I., Kieftenburg, A., Kriezi, E.,
654 2002. *Swan cycle iii version 40.11 user manual*. Delft University of Technol-
655 ogy, The Netherlands.

656 Jin, K.-R., Z.-G., J., 2001. Calibration and verification of a spectral wind-wave
657 model for lake okeechobee. *Ocean Eng.* 28, 571–584.

658 Joliffe, I., 2002. *Principal Component Analysis*. Springer.

659 Kalra, R., Deo, M., Kumar, R., Agarwal, V. K., 2005. Artificial neural net-
660 work to translate offshore satellite wave data to coastal locations. *Ocean*
661 *Engineering*, In Press, corrected proof available online 27 June.

662 Kan, A. R., Timmer, G., 1989. *Global Optimization: A Survey*, International
663 *Series of Numerical Mathematics*. Basel: Birkhauser Verlag.

664 Komar, P. D., Gaughan, M. K., 1972. Airy wave theory and breaker height
665 prediction. In: *Proc. of the 13th Conf. on Coastal Eng.* Vol. ASCE. pp.
666 405–418.

667 Kostanic, F. M. H. . I., 2001. *Principles of Neurocomputing for Science and*
668 *Engineering*. McGraw Hill.

669 Londhe, S. N., Deo, M. C., 2004. Artificial neural networks for wave propaga-
670 tion. *Journal of Coastal Research* 20 (4), 1061–1070.

671 Makarynsky, O., 2004. Improving wave predictions with artificial neural net-
672 works. *Ocean Engineering* 31 (5-6), 907–724.

673 Marquardt, D., 1963. An algorithm for least-squares estimation of nonlinear
674 parameters. *SIAM J. Appl. Math.* 11, 431–441.

675 Mase, H., Kitano, T., 1999. Prediction model for occurrence of impact wave
676 force. *Ocean Engineering* 26, 949–961.

677 Mase, H., Sakamoto, M., Sakai, T., 1995. Neural network for stability analysis
678 of rubble mound breakwaters. *Journal of Waterway, Coastal and Ocean*
679 *Engineering* 121 (6), 294–299.

680 Medina, J. R., 2005. Improving wave predictions with artificial neural net-
681 works, by o. makarynsky. *Ocean Engineering* 32 (1), 101–103.

682 Naithani, R., Deo, M. C., in print, 2005. Estimation of wave spectral shapes

683 using ann. *Advances in Engineering Software*.

684 Ou, S.-H., Liao, J.-M., Hsu, T.-W., Tzang, S.-Y., 2002. Simulating typhoon
685 waves by swan wave model in coastal waters of taiwan. *Ocean Eng.* 29,
686 947–971.

687 Rao, S., Mandal, S., 2005. Hindcasting of storm waves using neural networks.
688 *Ocean Engineering* 32, 667–684.

689 Ripley, B. D., 1996. *Pattern Classification and Neural Networks*. Cambridge.

690 Ris, R., Booij, N., Holthuijsen, L., 1998. A third-generation wave model for
691 coastal regions, part ii: verification. *J. Geophys. Res.* 104 (C4), 7649–7666.

692 Scotto, M., Soares, C. G., 2000. Modelling the long-term time series of sig-
693 nificant wave height with non-linear threshold models. *Coastal Engineering*
694 313–327.

695 Smith, W., Sandwell, D., 1997. Global seafloor topography from satellite al-
696 timetry and ship depth soundings. *Science* 277, 1956–1962.

697 Tolman, H. L., Krasnopolsky, V. M., Chalikov, D. V., 2005. Neural network
698 approximations for nonlinear interactions in wind wave spectra: direct map-
699 ping for wind seas in deep water. *Ocean Modelling* 8, 253–278.

700 Tsai, C., Shen, J. N., Kerh, T., 1999. Wave forecasting using neural network
701 model. In: Kumar, Topping (Eds.), *Artificial intelligence applications in civil*
702 *and structural engineering*. Civil-Comp. Press, pp. 123–130.

703 Tsai, C. P., Lee, T. L., 1999. Back-propagation neural network in tidal-level
704 forecasting. *Journal of Waterway, Coastal, and Ocean Engineering* 125 (4),
705 195–202.

706 Wood, D., Muttray, M., Oumeraci, H., 2001. The swan model used to study
707 wave evolution in a flume. *Ocean Eng.* 28, 805–823.

Table 1

Counts of surf reporter observations and total number of days monitored from 12/01/05 to 9/8/05. Observations were recorded more than once / day during more dynamic surf conditions.

	Region	Beach	Observations	Days
QLD		Surfer's Paradise	231	201
		Sunshine Coast	228	203
WA		Margaret River	229	202
		Perth	223	204
SA		Seaford	222	206
		Chiton Rocks	210	206
NNSW		Mid Coast	361	206
		North Coast	216	206
SNSW		Bondi	374	200
		Cronulla	362	204
		Manly	377	207
		Palm Beach	330	200
		South Coast	236	200
VIC		Woolamai	275	200
		Portsea	268	200
		Torquay	305	206
		Warrombool	243	200
			(N)	(N)

Table 2

Cross-correlation matrix and univariate mean and higher moments of significant wave heights H_s recorded at NOAA WW3 grid points from 12/01/05 to 9/8/05.

	NNSW	QLD	SA	SNSW	VIC	WA
North New South Wales (153.75 -31)	-					
Queensland (153.75, -28)	.75	-				
South Australia (136.25,-36)	-.23	-.24	-			
South New South Wales (151.25,-34)	.80	.49	-.24	-		
Victoria (143.75,-39)	-.16	-.23	.77	-.17	-	
Western Australia (115.00,-34)	-.02	.12	.21	-.11	-.02	-
mean \overline{H}_s	1.95	1.89	2.91	1.47	2.31	3.06
standard deviation $\sigma^2(H_s)$	0.85	0.65	1.01	0.78	0.84	1.20
skewness $\sigma^3(H_s)$	1.48	0.84	0.65	1.87	0.98	1.06
kurtosis $\sigma^4(H_s)$	6.02	4.26	3.02	8.32	3.88	4.23
(metres)						

Table 3

Mean and higher moments of surf reporter significant wave height H_s observations (in metres) from 12/01/05 to 9/8/05.

Region	Beach	\overline{H}_s	$\sigma^2(H_s)$	$\sigma^3(H_s)$	$\sigma^4(H_s)$
QLD	Surfer's Paradise	0.96	0.35	0.0076	0.0475
	Sunshine Coast	0.91	0.33	0.0086	0.0540
WA	Margaret River	2.02	0.66	0.0009	0.0177
	Perth	0.60	0.28	0.0092	0.0354
SA	Seaford	0.45	0.22	0.0068	0.0324
	Chiton Rocks	1.37	0.31	0.0003	0.0231
NNSW	Mid Coast	0.66	0.43	0.0128	0.0419
	North Coast	1.03	0.33	0.0116	0.0627
SNSW	Bondi	0.88	0.38	0.0132	0.0592
	Cronulla	0.88	0.39	0.0139	0.0608
	Manly	0.88	0.38	0.0126	0.0577
	Palm Beach	0.89	0.38	0.0128	0.0570
	South Coast	0.87	0.45	0.0081	0.0349
VIC	Woolamai	1.40	0.44	0.0002	0.0259
	Portsea	1.46	0.46	0.0034	0.0344
	Torquay	0.85	0.39	0.0085	0.0366
	Warranambool	0.89	0.41	0.0066	0.0360

Table 4

Cross-correlation matrix r of significant wave heights H_s at NOAA WW3 grid points from 12/01/05 to 9/8/05.

		Queensland			Western Aus.			South Aus.			N th NSW			S th NSW			Victoria		
		SP	SC	MR	PE	SF	CR	MC	NC	BI	CR	MY	PB	SC	WO	PO	TO	WL	
QLD	Surfer's Paradise SP	-																	
	Sunshine Coast SC	.96	-																
WA	Margaret River MR	.13	.08	-															
	Perth PE	.12	.09	.89	-														
SA	Seaford SF	.15	.15	.12	.14	-													
	Chiton Rocks CR	.12	.12	-.10	-.09	.45	-												
NNSW	Mid Coast MC	.41	.37	-.01	-.03	-.11	-.05	-											
	North Coast NC	.77	.71	.27	.24	.05	.03	.39	-										
SNSW	Bondi BI	.20	.11	.04	.01	-.31	-.23	.65	.36	-									
	Cronulla CR	.16	.07	.01	-.01	-.32	-.23	.64	.33	.98	-								
SNSW	Manly MY	.21	.11	.03	.01	-.31	-.23	.65	.37	.99	.97	-							
	Palm Beach PB	.21	.12	.04	.02	-.30	-.22	.66	.36	.99	.97	.99	-						
SNSW	South Coast SC	.12	.05	.06	.07	-.20	-.23	.59	.37	.74	.76	.74	.73	-					
	Woolamai WO	-.01	.01	-.20	-.22	.30	.62	-.10	-.07	-.12	-.12	-.12	-.12	-.18	-				
VIC	Portsea PO	-.01	-.01	-.13	-.19	.27	.59	-.13	-.06	-.10	-.11	-.11	-.10	-.17	.91	-			
	Torquay TO	-.00	.00	-.21	-.25	.15	.67	-.04	-.02	-.06	-.06	-.08	-.07	-.10	.83	.83	-		
VIC	Warranamboul WL	-.08	-.08	-.24	-.30	.10	.54	-.01	-.07	-.02	-.01	-.04	-.03	-.07	.71	.69	.79	-	

boldface indicates correlation $r > .30$

Table 5

Mean absolute error (MAE) in cm of NOAA WW3 derived significant height estimates with respect to surf reporter observations

Region	Beach	Baseline		Linear ANN		Non-linear ANN		Ensemble	SWAN						
		Raw	Scaled	Training		Validation									
				MAE	MAE	MAE	$\sigma(MAE)$	MAE		$\sigma(MAE)$	MAE	t^{**}			
QLD	Surfer's Paradise	91.77	23.30	23.30	17.70	0.12	17.84	1.32	14.93	0.96	16.12	1.05	13.60	3.22*	32.31
	Sunshine Coast	96.33	22.78	22.78	16.75	0.07	16.77	0.73	14.17	0.93	15.51	0.96	12.40	3.32*	
WA	Margaret River	102.89	36.64	36.64	34.20	0.17	34.74	1.30	23.75	1.46	26.69	1.64	21.34	12.17*	65.51
	Trigg Beach	244.11	13.32	13.32	13.46	0.13	13.80	0.55	11.15	0.67	12.05	0.89	10.16	5.25*	62.77
SA	Seaford	245.64	13.82	13.82	14.28	0.06	14.49	0.98	11.92	0.48	12.83	0.96	11.12	3.83*	15.20
	Chilton Rocks	152.84	21.15	21.15	20.56	0.04	20.64	0.52	16.17	0.76	17.42	0.76	15.12	11.02*	40.93
NNSW	Mid Coast	127.92	22.91	22.91	23.42	0.06	22.97	1.19	18.65	1.03	19.62	1.13	17.19	6.46*	
	North Coast	90.93	21.41	21.41	20.28	0.22	19.98	0.67	17.13	1.35	18.15	1.17	15.08	4.30*	
	Bondi	59.54	19.47	19.47	21.64	0.13	21.73	1.31	18.12	0.52	19.41	1.04	16.96	4.38*	
	Cronulla	60.26	19.94	19.94	22.22	0.15	21.85	1.09	18.76	0.75	20.25	0.93	17.26	3.53*	
SNSW	Manly	60.18	19.86	19.86	21.71	0.09	21.40	1.50	18.41	0.65	19.49	1.19	17.33	3.15*	50.06
	Palm Beach	58.97	19.72	19.72	21.96	0.19	22.26	0.88	17.69	0.65	19.75	1.34	16.42	4.95*	
	South Coast	64.73	28.06	28.06	26.97	0.11	27.45	1.33	22.14	1.70	24.20	1.53	20.56	5.06*	
	Woolamai	89.94	29.62	29.62	27.88	0.09	28.30	1.05	23.93	0.86	25.64	1.47	22.70	4.67*	38.72
	Portsea	83.22	28.81	28.81	28.61	0.14	28.72	1.44	24.49	0.89	26.61	1.78	22.62	2.92*	
VIC	Torquay	143.57	24.18	24.18	23.73	0.13	24.34	1.15	19.16	1.00	21.03	1.45	17.83	5.66*	34.98
	Warranamboul	141.49	27.93	27.93	26.00	0.11	25.96	1.07	23.90	0.72	25.29	1.23	22.78	1.30	
* indicates 1-tailed t-statistic significant (N=5, $p < 0.01$) ** t-statistic: two sample, one-tailed t-test testing $\mathbf{H}(\frac{MAE_{old}^{val}}{MAE_{lin}^{val}})$															

* indicates 1-tailed t-statistic significant ($N=5, p < 0.01$) ** t -statistic: two sample, one-tailed t-test testing $\mathbf{H}(\overline{MAE}_{nl}^{val} < \overline{MAE}_{lin}^{val})$

Table 6

Correlation R of NOAA WW3 derived surf height estimates with surf reporter observations

Region	Beach	Baseline	Linear ANN		Non-linear ANN		Ensemble	$\mathbf{H}(\bar{r}_{nl}^{val} > \bar{r}_{lin}^{val})$ t -statistic	SWAN					
		Raw	Scaled	Training	Validation	Training				Validation				
			\bar{r}	\bar{r}	\bar{r}	$\sigma(r)$	\bar{r}			$\sigma(r)$	\bar{r}	$\sigma(r)$		
QLD	Surfer's Paradise	.49	.49	.73	.008	.71	.036	.83	.027	.79	.037	.86	4.84*	.72
	Sunshine Coast	.46	.46	.72	.008	.71	.026	.83	.025	.80	.029	.87	7.42*	
WA	Margaret River	.77	.77	.79	.005	.78	.018	.88	.011	.86	.022	.90	8.23*	.73
	Trigg Beach	.79	.79	.80	.015	.80	.028	.87	.018	.85	.023	.89	4.82*	.72
SA	Seaford	.67	.67	.63	.030	.61	.045	.75	.020	.70	.027	.78	5.81*	.63
	Chiton Rocks	.58	.58	.62	.008	.64	.029	.77	.021	.74	.028	.81	8.63*	.40
NNSW	Mid Coast	.72	.72	.74	.004	.73	.040	.83	.021	.80	.039	.86	3.90*	
	North Coast	.34	.34	.52	.014	.52	.042	.74	.058	.68	.070	.81	6.28*	
SNSW	Bondi	.81	.81	.75	.005	.76	.019	.84	.0093	.82	.021	.86	6.86*	
	Cronulla	.80	.80	.75	.005	.75	.026	.84	.011	.79	.016	.87	4.63*	
	Manly	.80	.80	.75	.004	.75	.036	.84	.0093	.81	.025	.86	4.55*	.76
	Palm Beach	.81	.81	.75	.006	.75	.035	.85	.011	.81	.036	.87	4.15*	
	South Coast	.69	.69	.72	.006	.70	.047	.80	.029	.76	.032	.84	3.16*	
VIC	Woolamai	.67	.67	.69	.003	.68	.033	.78	.018	.74	.034	.80	3.97*	.62
	Portsea	.70	.70	.69	.007	.69	.029	.78	.015	.74	.030	.81	3.53*	
	Torquay	.68	.68	.70	.007	.71	.016	.81	.019	.78	.030	.84	7.35*	.70
	Warranambool	.59	.59	.63	.004	.64	.040	.71	.022	.69	.031	.75	2.91*	

* indicates 1-tailed t-statistic significant ($N=5$, $p < .01$)

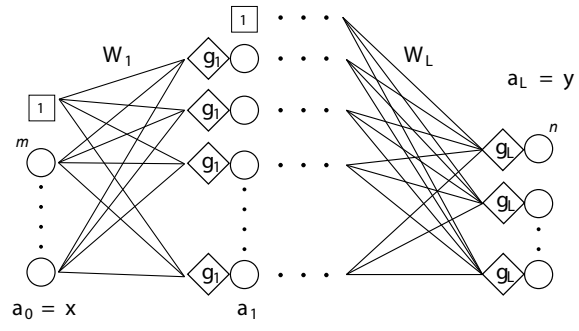


Fig. 1.

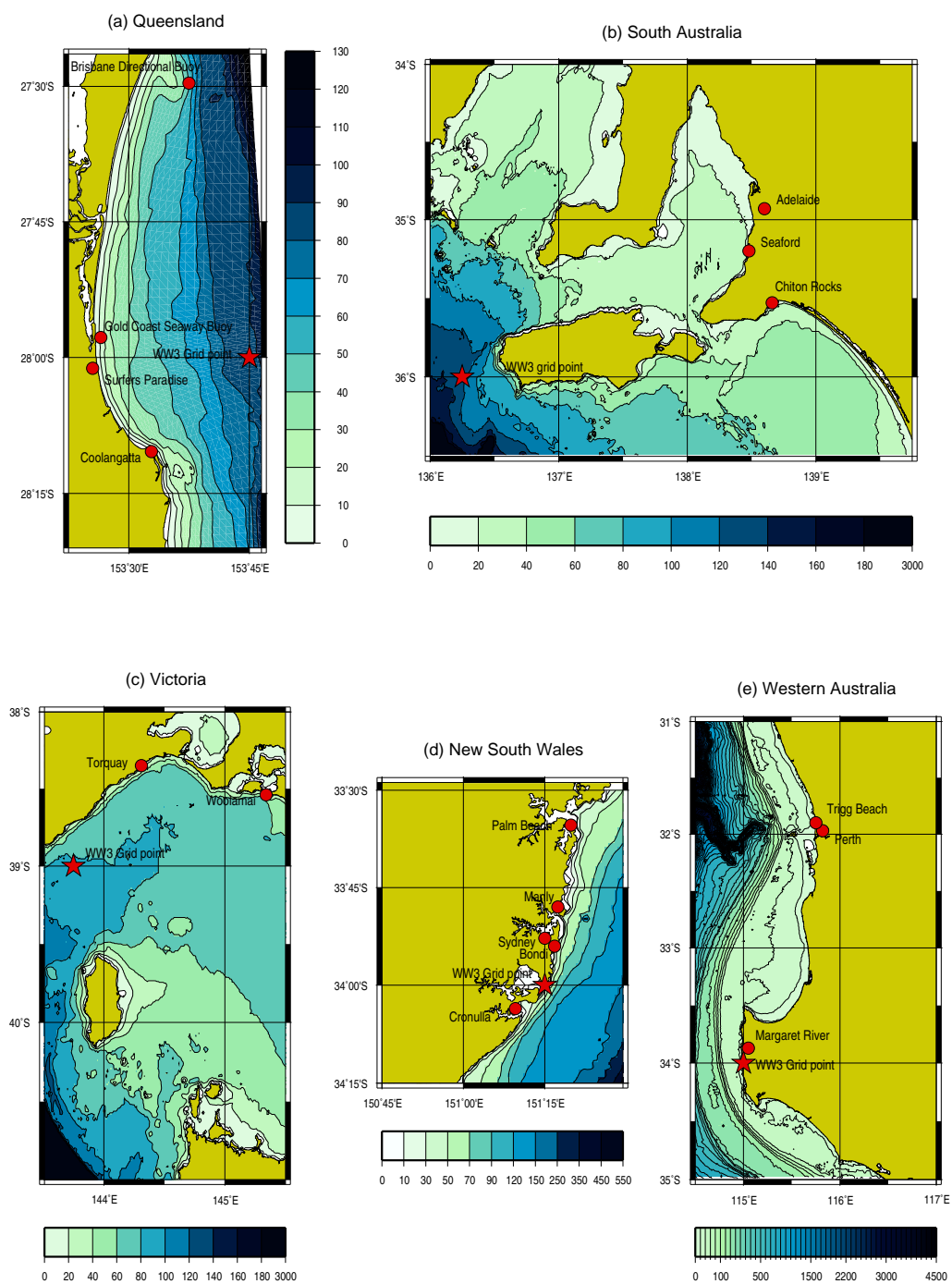


Fig. 2.

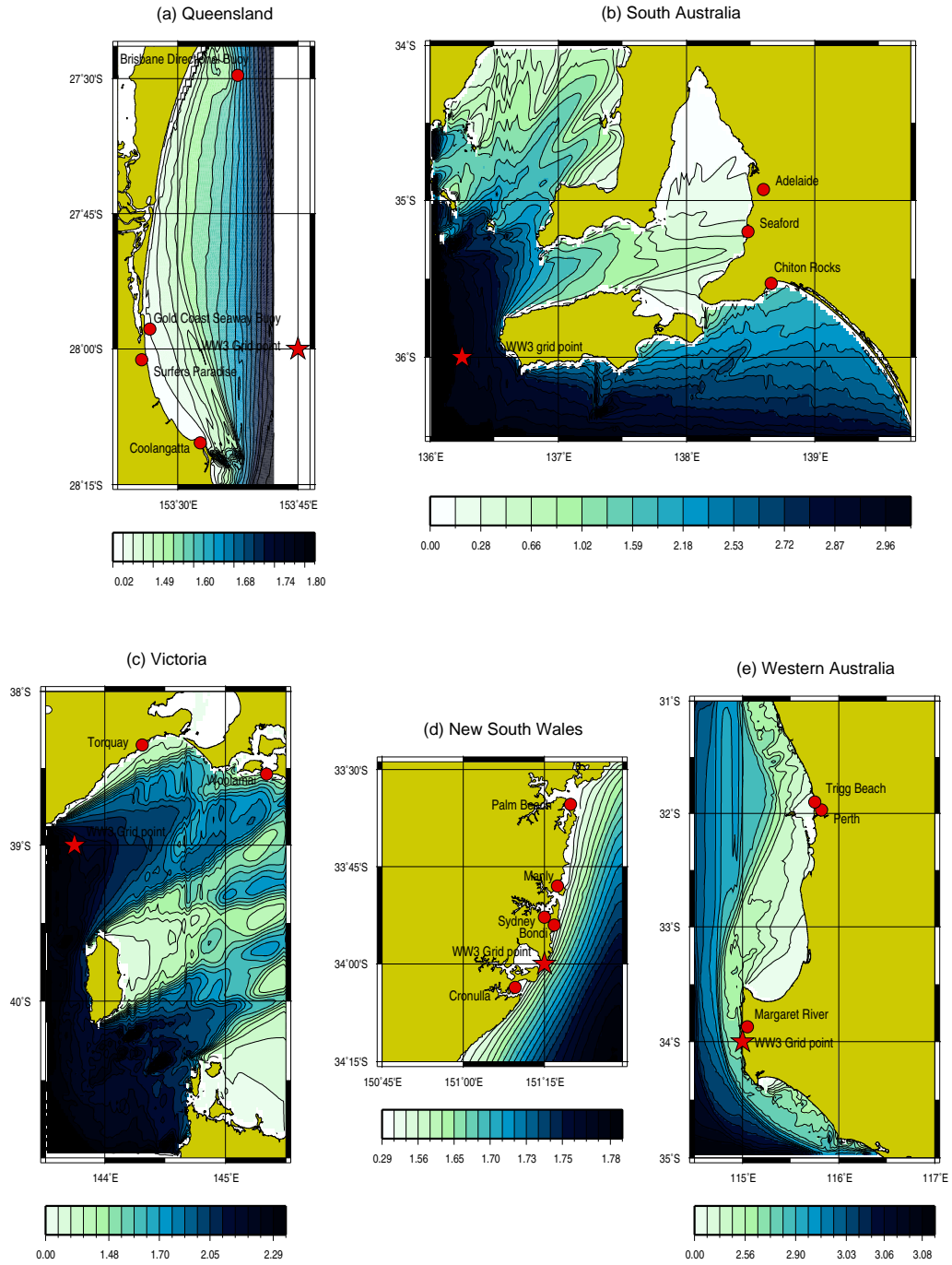


Fig. 3.

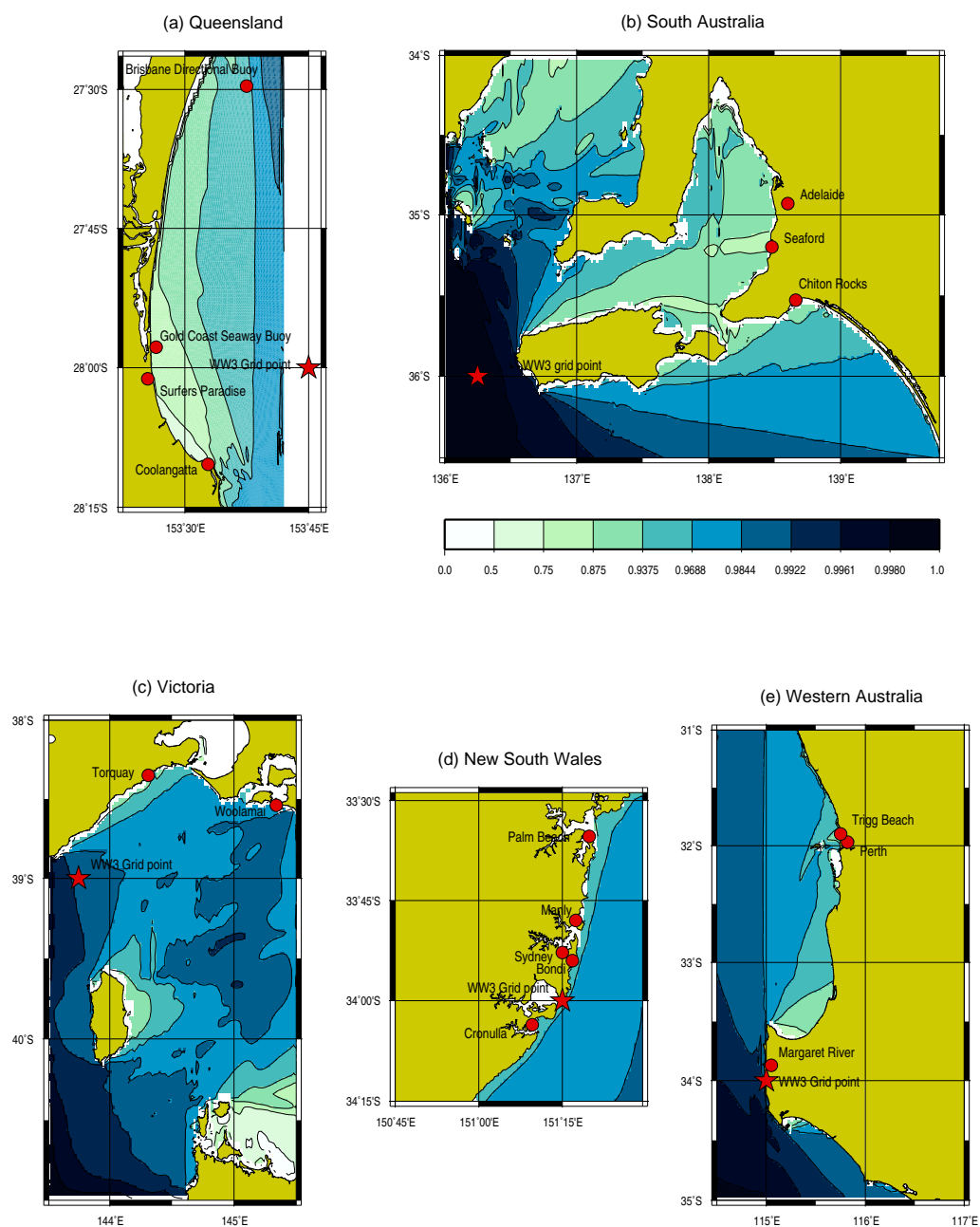


Fig. 4.

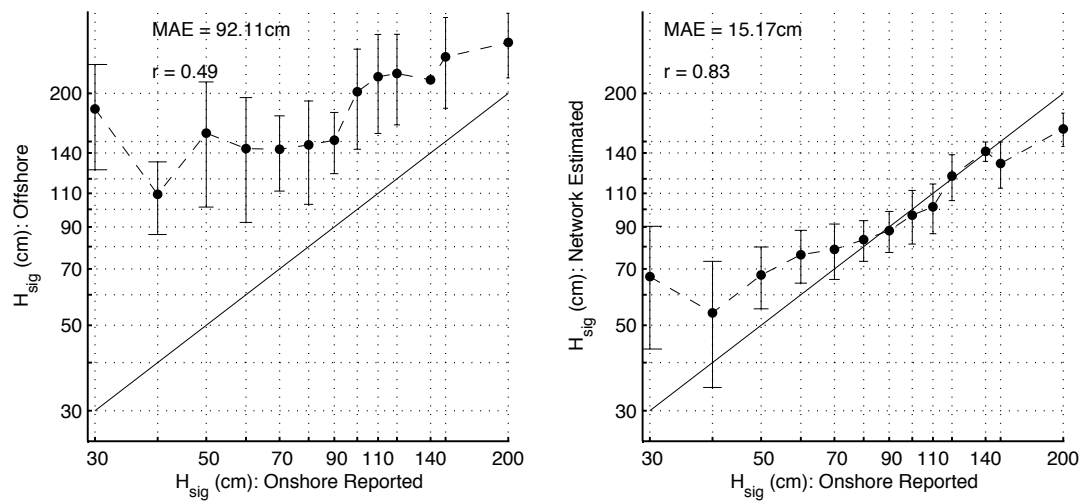
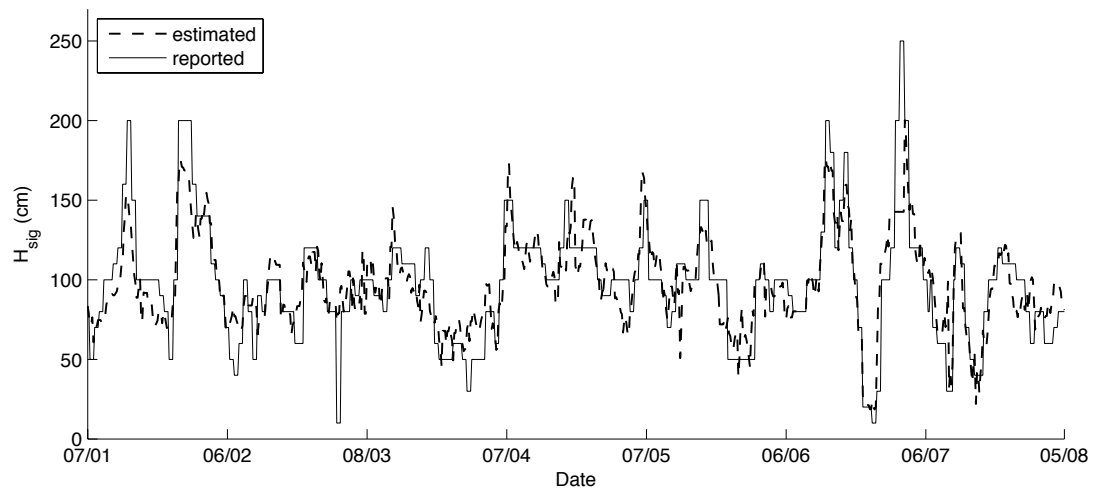
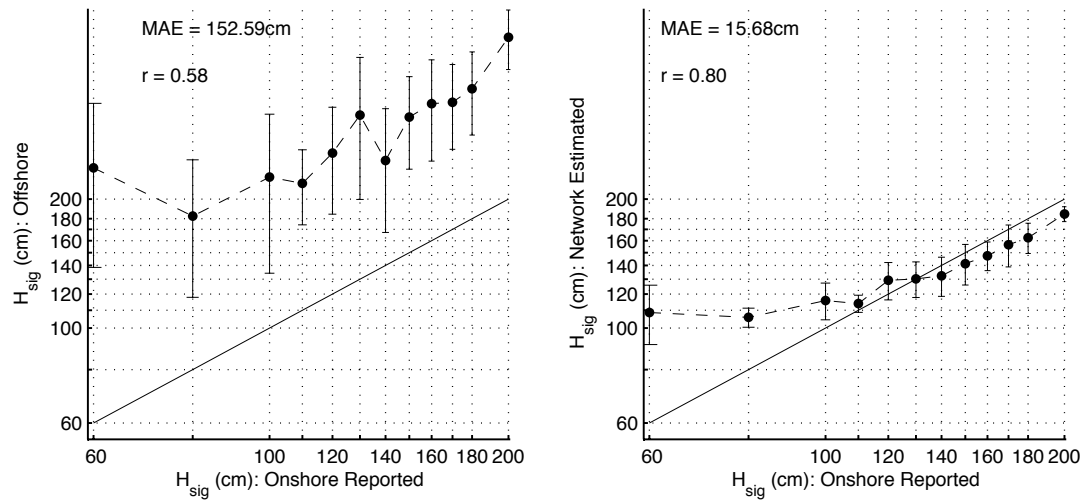
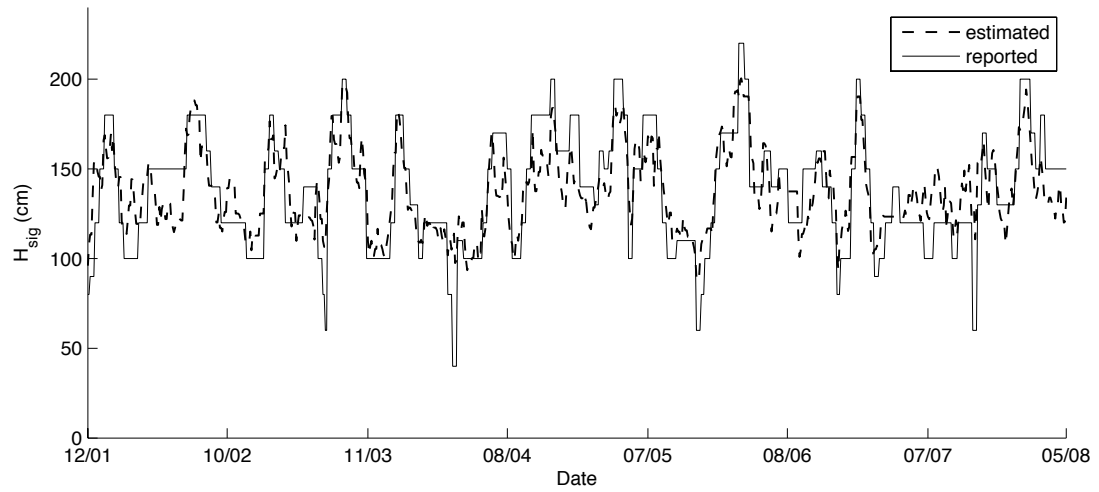
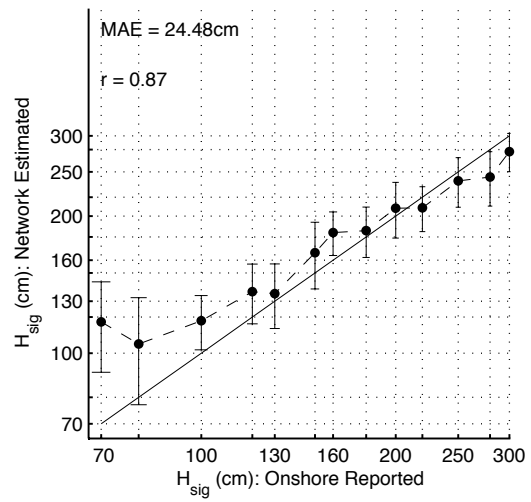
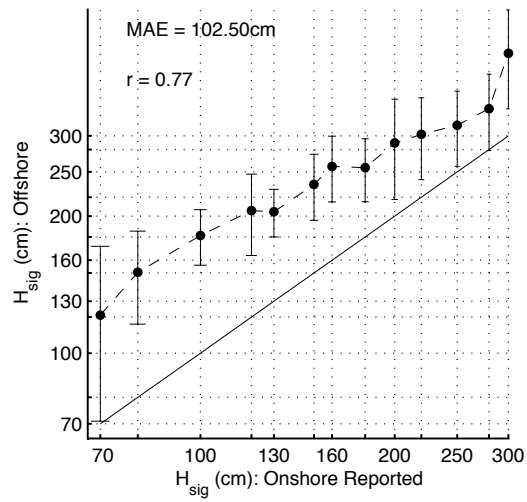
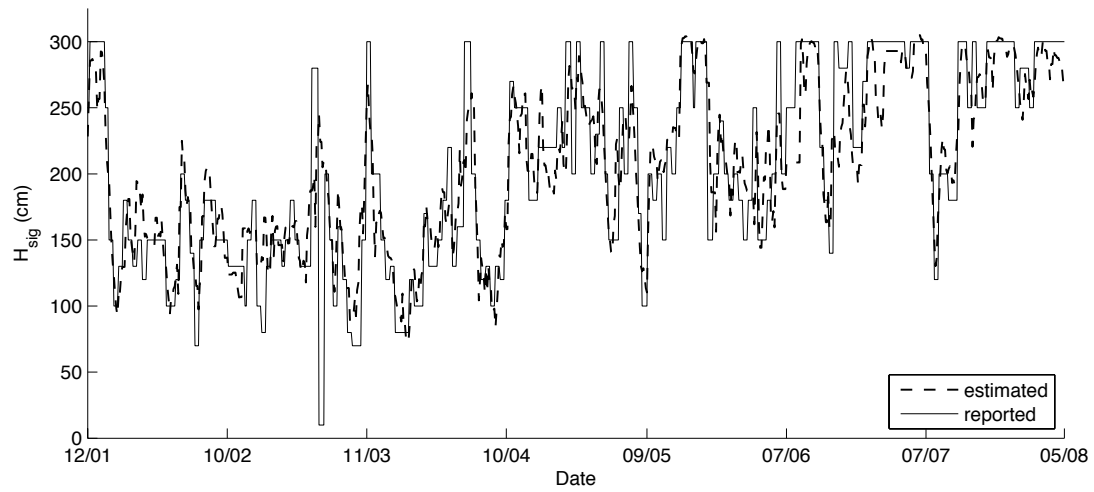


Fig. 5.



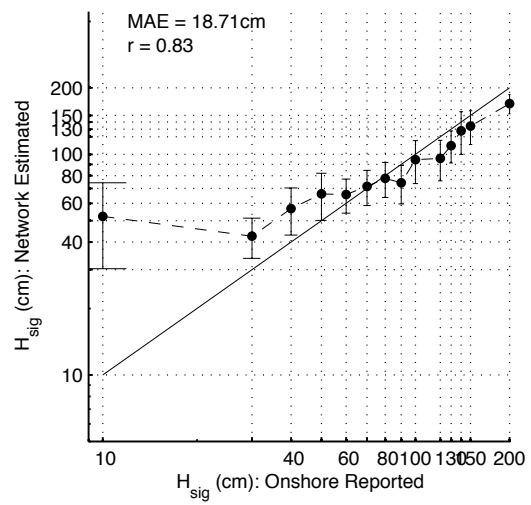
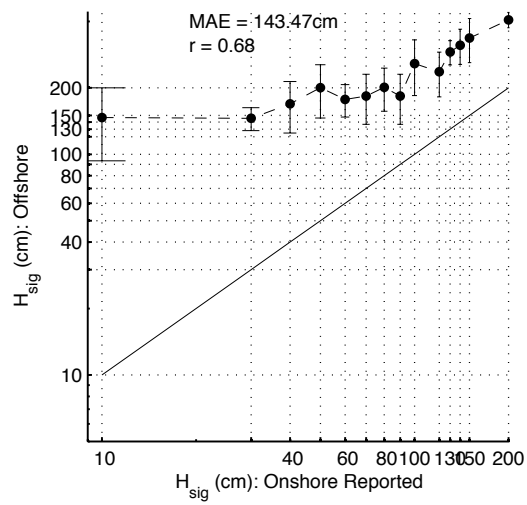
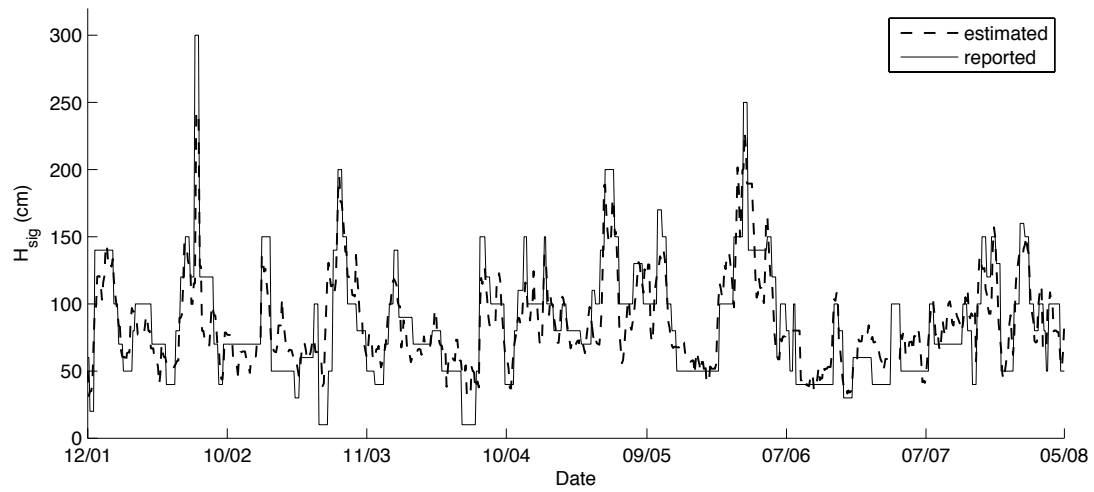
(a) Chiton Rocks

Fig. 6.



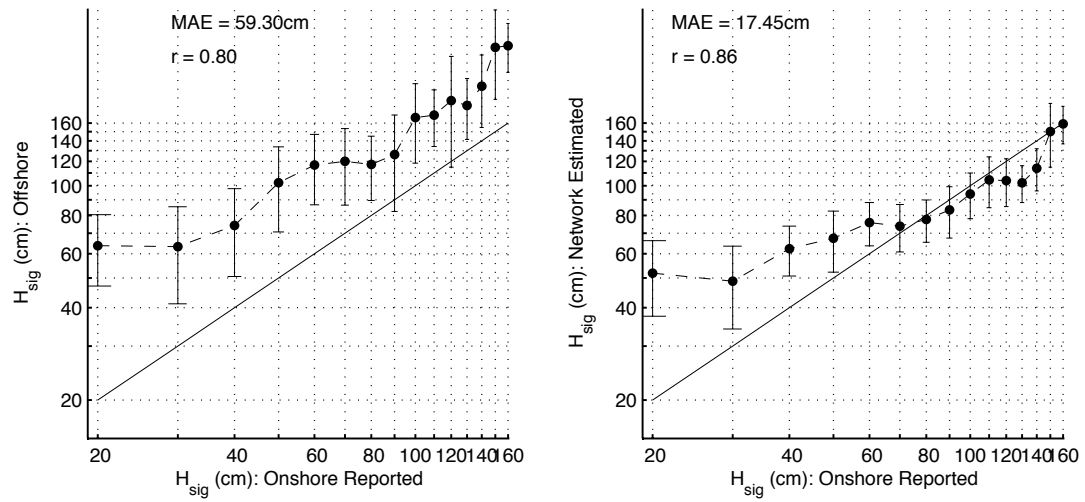
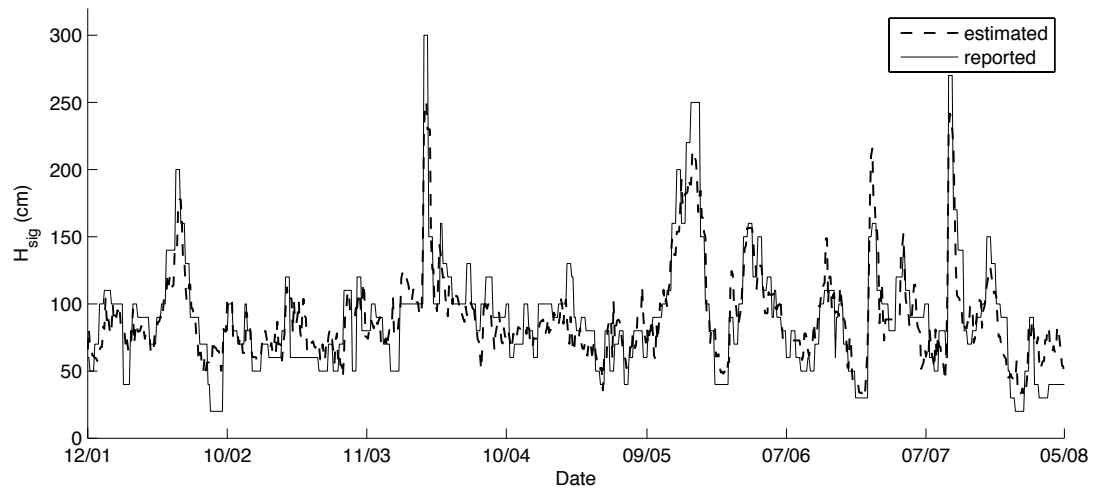
(a) Margaret River

Fig. 7.



(a) Torquay

Fig. 8.



(a) Palm Beach, Sydney

Fig. 9.

708 Fig. 1 Standard feed-forward ANN architecture

709

710 Fig. 2 Bathymetry for five of the study regions

711

712 Fig. 3 Mean significant wave height as estimated by the SWAN model when
713 driven by NWW3 over the study period

714

715 Fig. 4 Normalised correlation coefficient r between the NOAA driving in-
716 put and SWAN output significant wave height over the study period. Lighter
717 shades indicate local regions affected by more complex wave transformations:
718 i.e. a greater proportion of observed wave height variability is not linearly re-
719 lated to the wave height at the model boundaries

720

721 Fig. 5 ANN estimation performance at Surfer’s Paradise, Queensland

722

723 Fig. 6 ANN estimation performance on South Australian beaches

724

725 Fig. 7 ANN estimation performance on Western Australian beaches

726

727 Fig. 8 ANN estimation performance on Victorian beaches

728

729 Fig. 9 ANN estimation performance on New South Wales beaches